



# MEMORANDUM

TO: City of Keizer Engineering Department  
FROM: Mike Towle, PE  
DATE: December 8, 2020  
SUBJECT: Chemawa Station Stormwater Design – Chick-fil-A Update

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This memo outlines the proposed changes to the stormwater management facilities at the Chemawa Station development, originally designed by Westech Engineering in 2019.

The original site layout shows two buildings on the west side of the development, with a shared parking area between them. This site layout was approved in early 2020, and the mass grading and utility work was done during the summer. As builds were submitted for this work on October 1, 2020 (See Attachments: Pages from Phase II: Final Grading, Drainage & Utilities Chemawa Station – “Area D”).

The proposed Chick-fil-A (CFA) site plan has modified the original site plan for the west side of the Chemawa Station development (referred to as “Basin West” by Westech). The building originally shown on the northwest side of the site has been moved to the west end of Basin West. The building originally shown on the southwest side of the site has been removed (See Attachments: Figure 2 – Proposed Conditions).

Due to this updated site plan, the previously installed stormwater treatment and detention facilities need to be relocated since they are now conflicting with the proposed building location. The stormwater design was checked to confirm the facility sizing is correct per the modified site plan. The original storm report was provided by Westech Engineering and was used to compare the new site conditions to the original design for this area.

The original design for the treatment and detention systems accounts for 1.76 ac of impervious area in Basin West. The proposed changes to the original design due to the new CFA layout results in 1.44 ac of impervious area within Basin West.

The new CFA layout has less impervious area than the original site layout, which means the water quality flow from the Basin West area decreased and the release rates from the detention system decreased. Therefore, the treatment and detention systems are adequately sized, and no additional filter cartridges or chambers are needed.

The attached exhibit shows the proposed new layout for the filter vault and chamber system. Please note, the final layout of these facilities is to be determined during the construction phase.

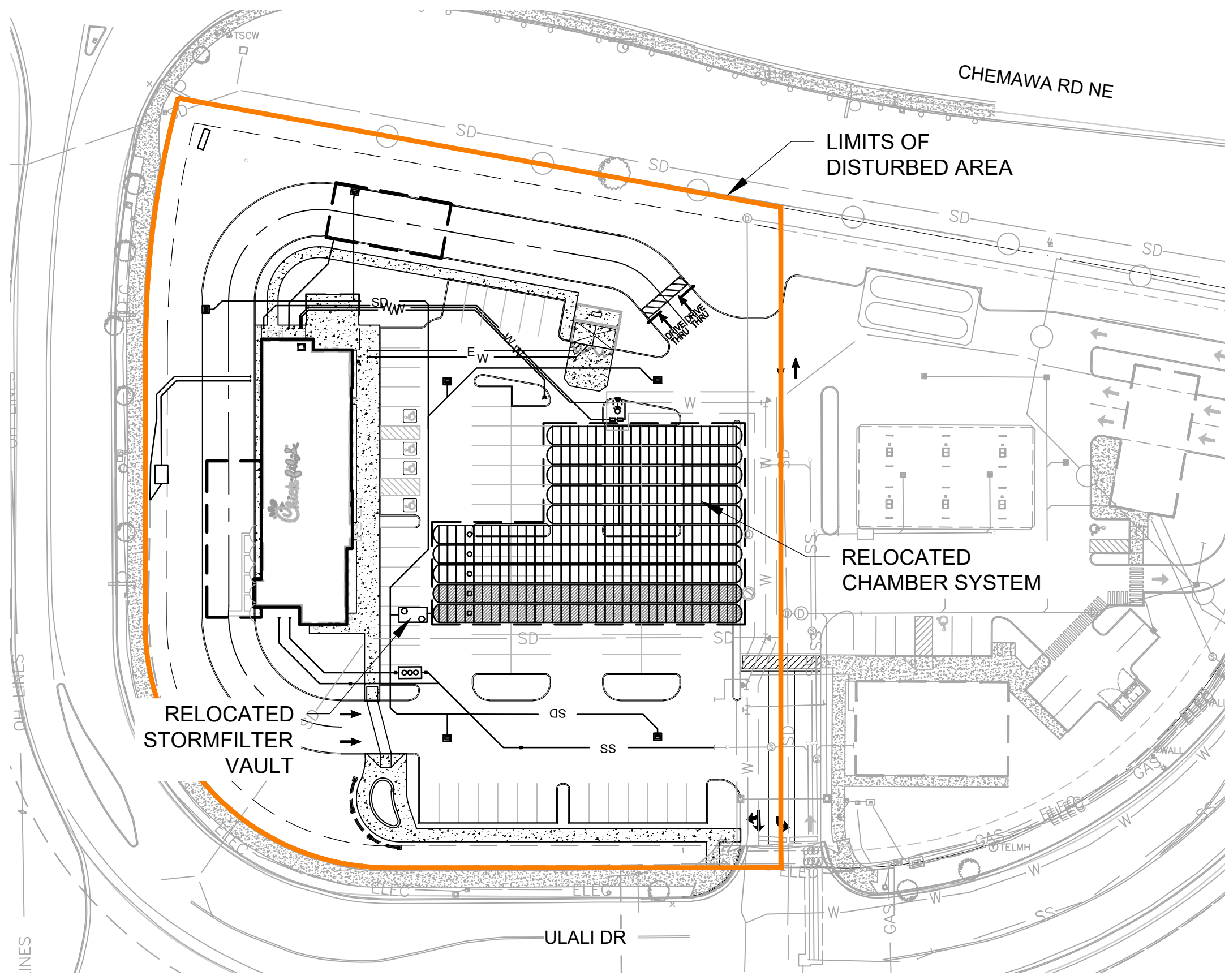
Please feel free to contact me with any questions.

Mike Towle, PE

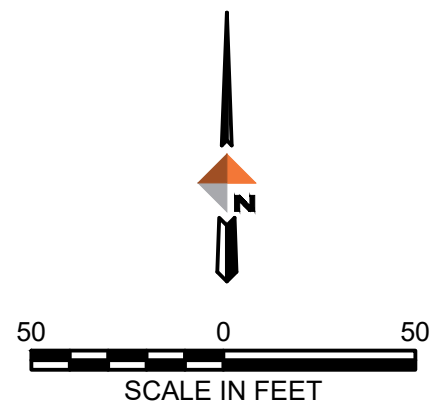
Attachments:

- Figure 2: Proposed Conditions
- Pages from Phase II: Final Grading, Drainage & Utilities Chemawa Station “Area D” – October 2020.
- Pages from Stormwater Calculations Chemawa Station – August 2019.

I:\BIL-FS\BIL-projects\2214707-0140Study\Drain\cad\14707-proposed.dwg PLOT DATE 2020-12-8 12:39 SAVED DATE 2020-11-10 17:40 USER: mjlllette



Site Condition	Impervious Area (ac)	Pervious Area (ac)	Total Area (ac)
Proposed	1.44	0.70	2.14



**DOWL**  
 WWW.DOWL.COM  
 720 SW Washington Street, #750  
 Portland, Oregon 97205  
 971-280-8641

**CHICK-FIL-A KEIZER  
 PROPOSED CONDITIONS  
 KEIZER, OREGON**

PROJECT	14707
DATE	12/08/2020

**FIGURE 2**

DRAWINGS FOR:

# PHASE II: FINAL GRADING, DRAINAGE & UTILITIES CHEMAWA STATION-"AREA D" ULALI DR, KEIZER, OR

FOR:

CHEMAWA STATION, LLC  
9400 SW BEAVERTON-HILLSDALE HWY, STE. 131-A  
BEAVERTON, OREGON 97005

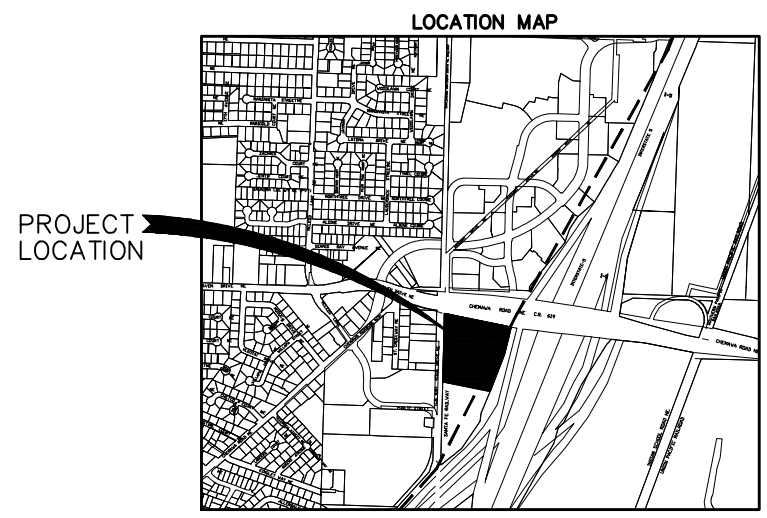


Know what's below.  
Call before you dig.

**WE**  
WESTTECH ENGINEERING, INC.  
CONSULTING ENGINEERS AND PLANNERS  
3841 Fairview Industrial Dr., S.E., Suite 100, Salem, OR 97302  
Phone: (503) 868-2474 Fax: (503) 388-3988  
E-mail: westtech@westtech-eng.com  
WE #: 3150.0000.0



Chemawa Station - Area 'D'  
Ulali Drive | Keizer, Oregon  
Chemawa Station LLC



PROJECT NO. \_\_\_\_\_

DRAWN BY : AK

CHECKED BY : JW

DATE : 6-12-19

REVISION :

1. 7.22.19 AK PER CITY COMMENTS
2. 10.6.19 AK PER CITY COMMENTS
3. 1.22.20 AK PER OLDC COMMENTS
4. 3.25.20 AK COORDINATION W/ CFA
5. 4.15.20 AK PER CITY COMMENTS
- 6.

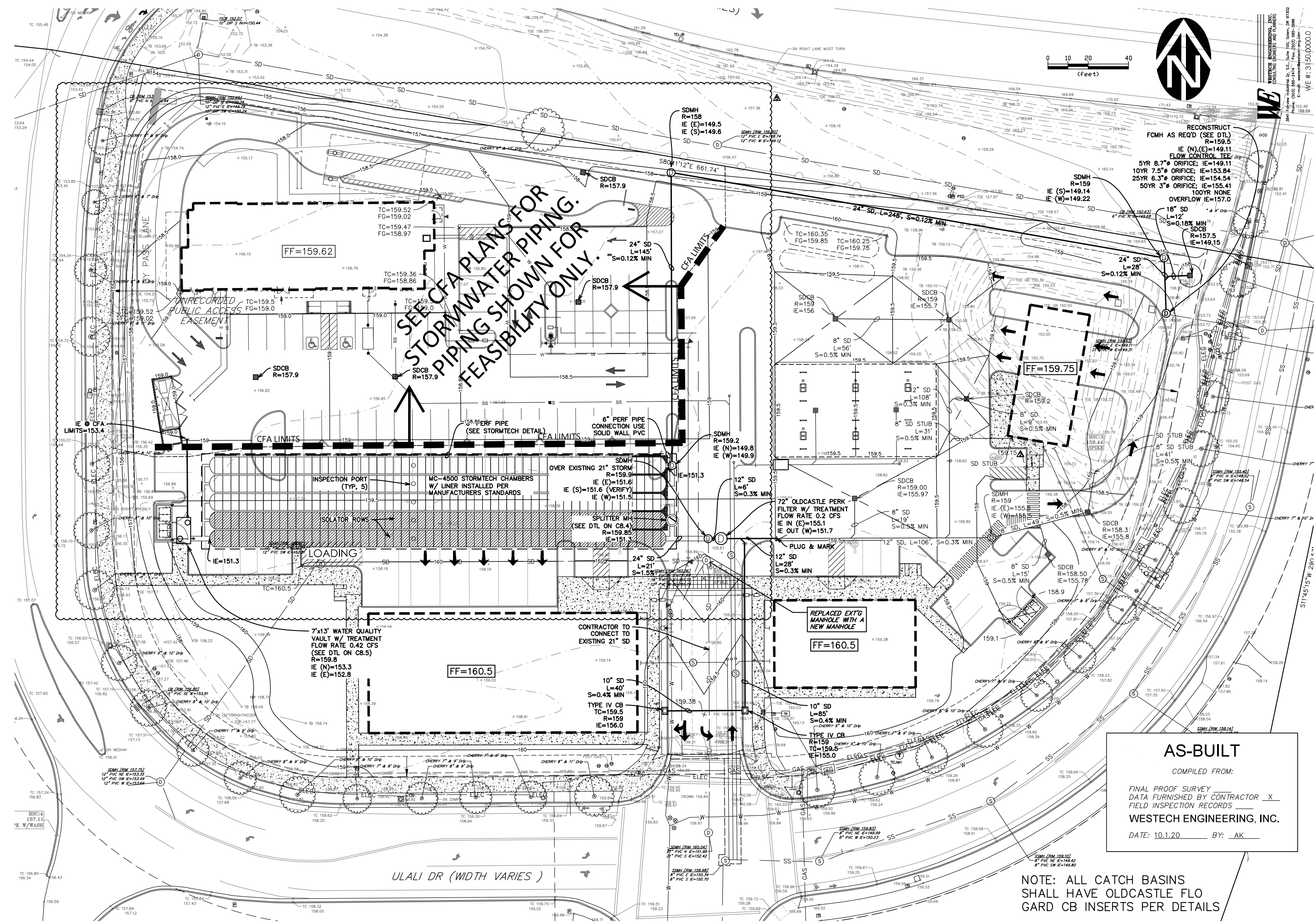
c1.0 cover

SHEET INDEX			
SHT NO	DESCRIPTION	SHT NO	DESCRIPTION
C0.0	COVER SHEET, LOCATION MAP, & SHEET INDEX	C8.6	CONSTRUCTION DETAILS
C1.0	SURVEY	C8.7	CONSTRUCTION DETAILS
C2.0	EROSION CONTROL & DEMOLITION PLAN		
C2.1	EROSION CONTROL NOTES		
C2.2	EROSION CONTROL DETAILS		
C2.3	EROSION CONTROL DETAILS		
C2.4	POST-DEVELOPMENT EROSION CONTROL PLAN		
C3.0	OVERALL SITE PLAN		
C4.0	OVERALL GRADING PLAN		
C4.1	OVERALL DRAINAGE PLAN		
C5.0	OVERALL UTILITY PLAN		
C6.0	OVERALL SURFACING PLAN		
C7.0	CONSTRUCTION NOTES		
C8.0	CONSTRUCTION DETAILS		
C8.1	CONSTRUCTION DETAILS		
C8.2	CONSTRUCTION DETAILS		
C8.3	CONSTRUCTION DETAILS		
C8.4	CONSTRUCTION DETAILS		
C8.5	CONSTRUCTION DETAILS		

**AS-BUILT**  
COMPILED FROM:  
FINAL PROOF SURVEY \_\_\_\_\_  
DATA FURNISHED BY CONTRACTOR  X   
FIELD INSPECTION RECORDS \_\_\_\_\_  
**WESTTECH ENGINEERING, INC.**  
DATE: 10.1.20 BY: AK

**BSAA**  
ARCHITECTURE & PLANNING  
BENNER STANGE ASSOCIATES ARCHITECTS, INC.  
THE WATERMAN BUILDING  
80 SE MADISON STREET  
SUITE 430  
PORTLAND, OR 97214  
503-670-0234  
FAX 503-670-0235  
bsa@bsaarch.com

COVER  
**C0.0**



WESTTECH ENGINEERING, INC.  
CONSULTING ENGINEERS AND PLANNERS  
3901 N. Holladay Blvd., Suite 100, Salem, OR 97302  
Phone: (503) 882-2474 Fax: (503) 881-3888  
E-mail: westtech@westtech-inc.com  
WE # 3150,0000.0



SEE CFA PLANS FOR  
STORMWATER PIPING.  
PIPING SHOWN FOR  
FEASIBILITY ONLY.

RECONSTRUCT  
FCMH AS REQ'D (SEE DTL)  
R=159.5  
IE (N),(E)=149.11  
FLOW CONTROL TEE:  
5YR 8.7" ORIFICE: IE=149.11  
10YR 7.5" ORIFICE: IE=153.84  
25YR 6.3" ORIFICE: IE=154.54  
50YR 3" ORIFICE: IE=155.41  
100YR NONE  
OVERFLOW IE=157.0

18" SD  
L=12'  
S=0.18% MIN  
SDCB  
R=157.5  
IE=149.15

24" SD  
L=28'  
S=0.12% MIN

24" SD  
L=145'  
S=0.12% MIN

24" SD  
L=248'  
S=0.12% MIN

24" SD  
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S=0.3% MIN

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Chemawa Station - Area 'D'  
Ulali Drive | Keizer, Oregon  
Chemawa Station LLC

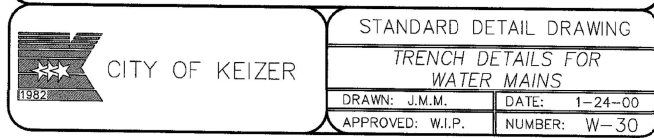
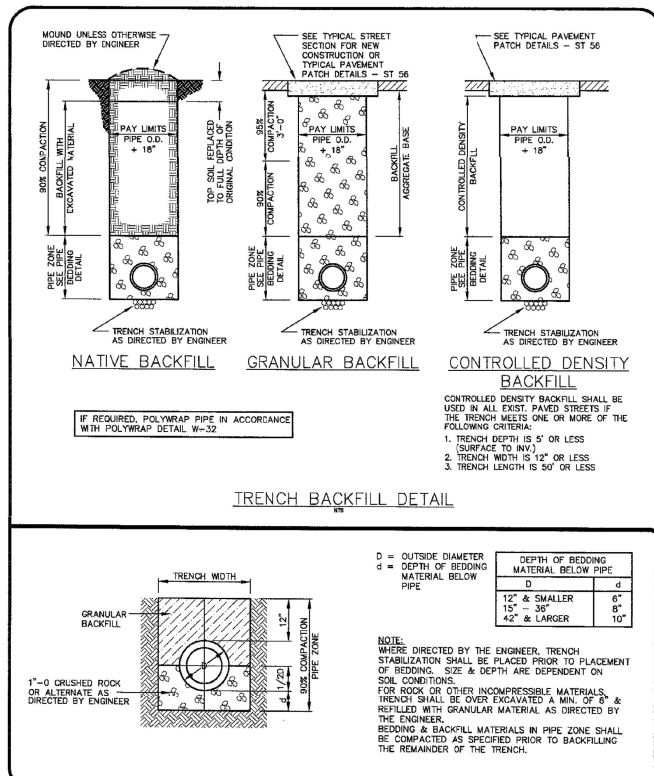
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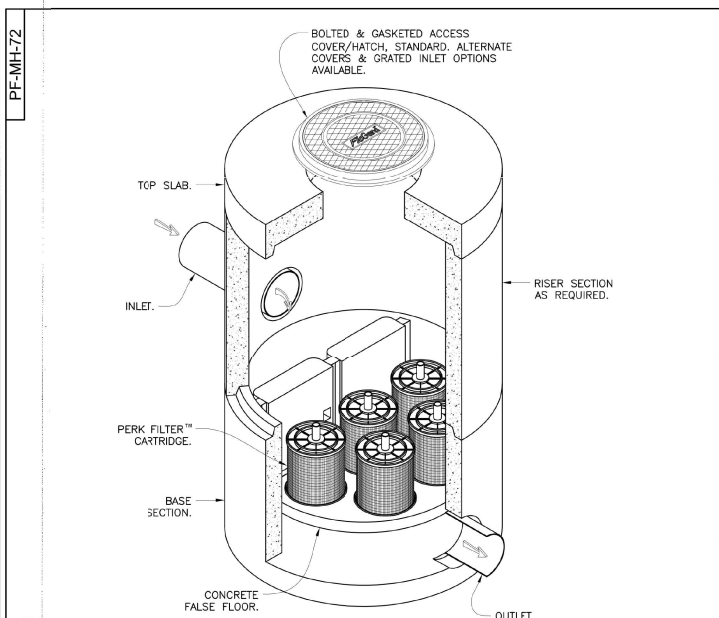
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DATA FURNISHED BY CONTRACTOR X  
FIELD INSPECTION RECORDS  
WESTTECH ENGINEERING, INC.  
DATE: 10.1.20 BY: AK

NOTE: ALL CATCH BASINS  
SHALL HAVE OLDCASTLE FLO  
GARD CB INSERTS PER DETAILS

OVERALL DRAINAGE  
PLAN  
**C4.1**



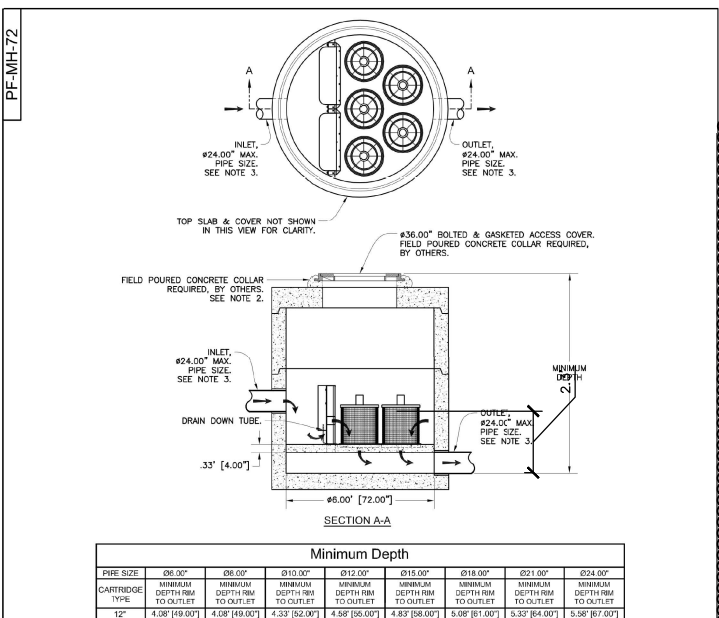
CITY OF KEIZER  
 DRAWN: J.M.M. DATE: 1-24-00  
 APPROVED: W.I.P. NUMBER: W-30



- Notes:
1. Precast concrete structure shall be manufactured in accordance with ASTM Designation C478.
  2. Filter system shall be supplied with traffic rated (H20) bolted & gasketed Ø36" circular access covers with risers as required. Field poured concrete collar required, by others.
  3. Inlet & outlet pipe(s) are to be Ø24.00" maximum. Inlet pipes must enter the structure in the inlet bay.
  4. Inlet chamber shall be supplied with drain-down device designed to remove standing water between storm events.
  5. Minimum separation between invert in & invert out is outlet pipe diameter plus 4.00".
  6. For depths less than specified minimums contact Oldcastle® Stormwater Solutions for engineering assistance.

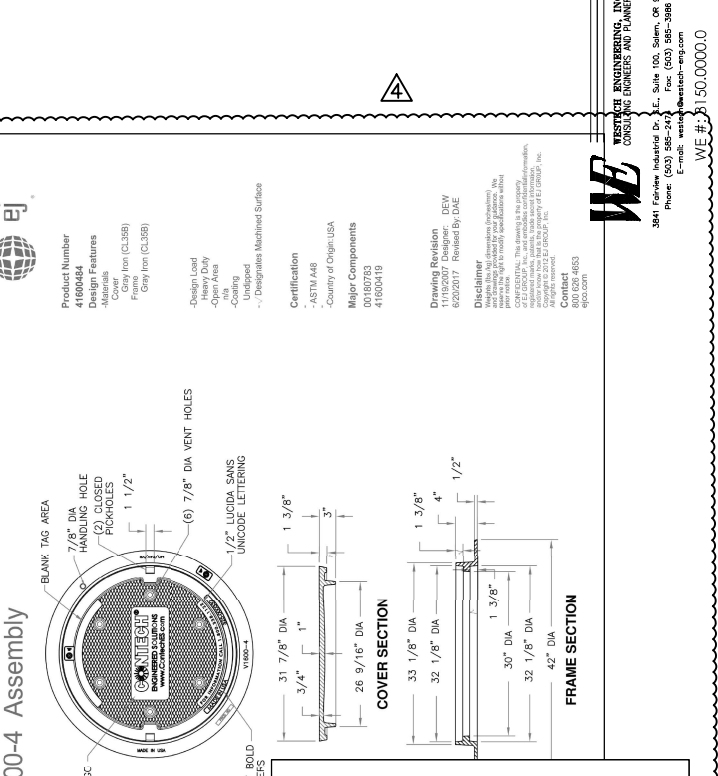
**Perk Filter™**  
 Ø72.00" Manhole  
 One to Five Cartridges / Stacks

**Oldcastle®**  
 Stormwater Solutions  
 7201 Southgate Plaza, Suite 200 | Littleton, CO 80120 | Ph: 800.579.8819 | oldcastlestormwater.com

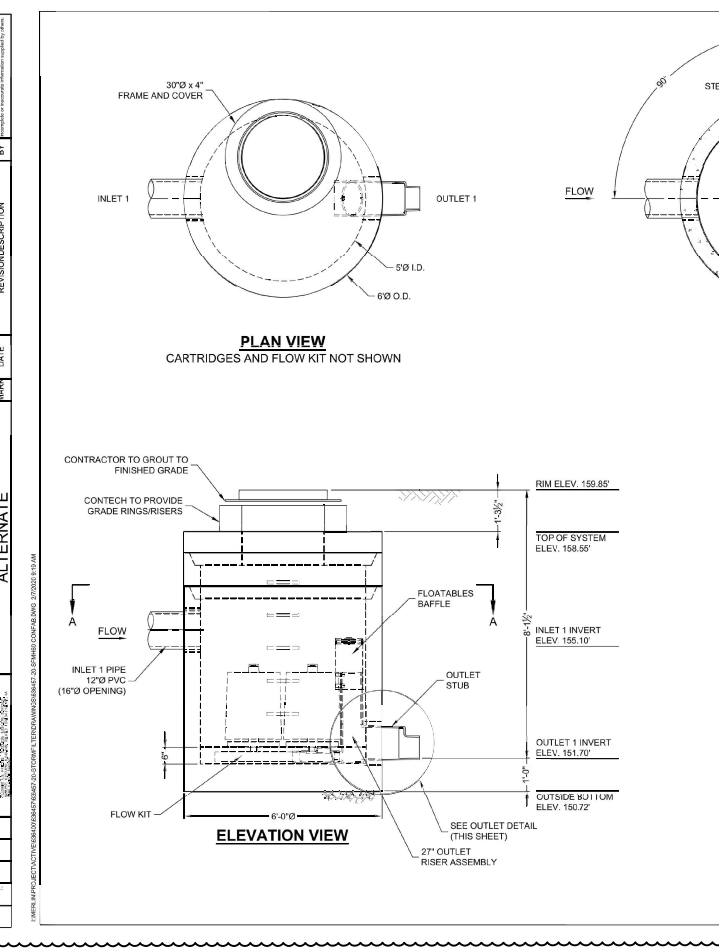
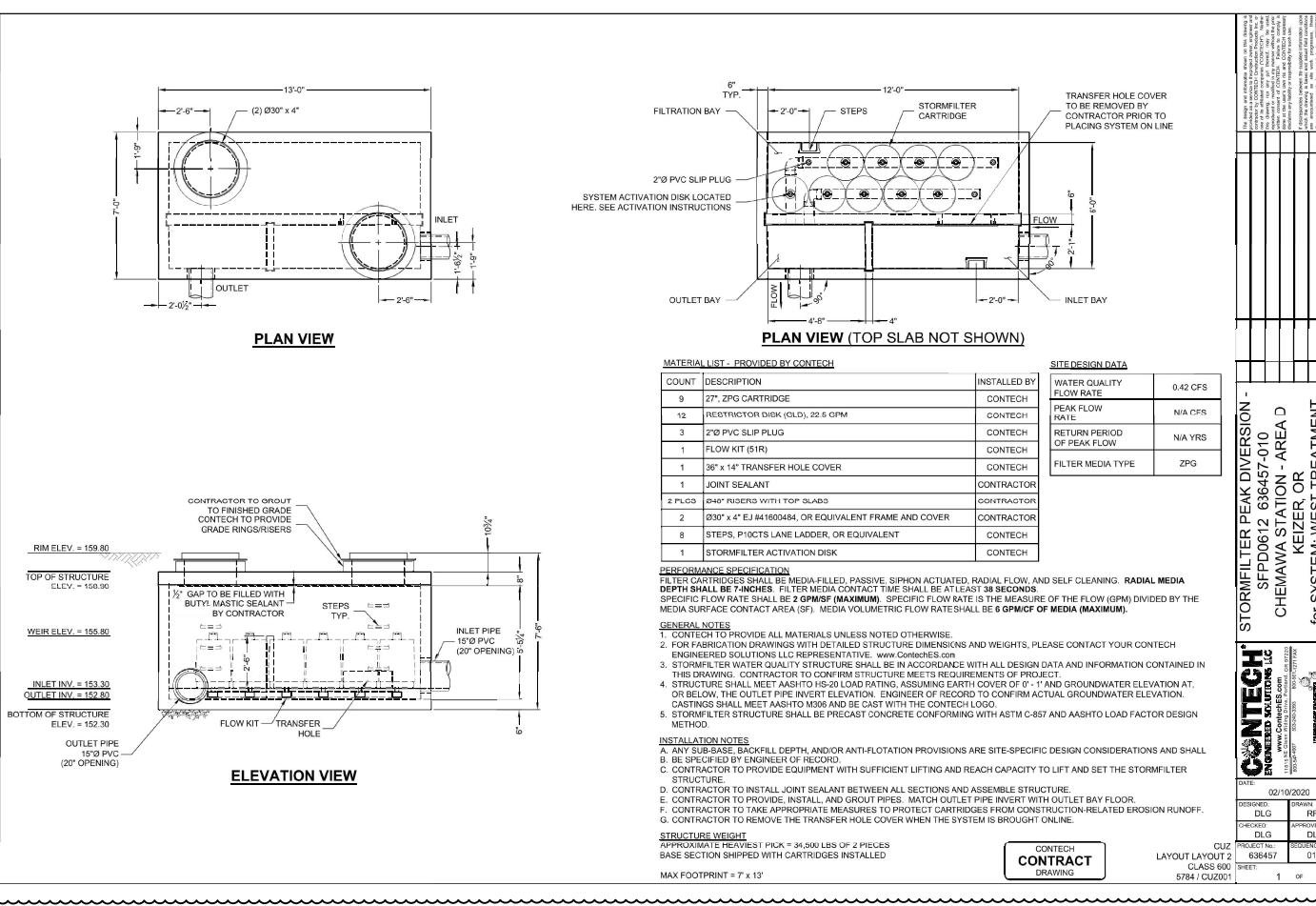


**Perk Filter™**  
 Ø72.00" Manhole  
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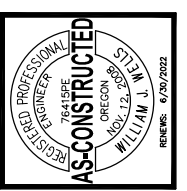
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**WESTECH ENGINEERING, INC.**  
 DATE: 10.1.20 BY: AK



**CONTECH ENGINEERED SOLUTIONS LLC**  
 STORMFILTER PEAK DIVERSION - AREA D  
 CHEMAMA STATION - AREA D  
 for SYSTEM: WEST TREATMENT ALTERNATE

**CONTECH ENGINEERED SOLUTIONS LLC**  
 STORMFILTER PEAK DIVERSION - AREA D  
 CHEMAMA STATION - AREA D  
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Chemama Station - Area 'D'  
 Ulali Drive | Keizer, Oregon  
 Chemama Station LLC

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 80 SE MADISON STREET  
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 PORTLAND, OR 97214  
 503-670-0234  
 fsa@bsaarch.com

CONSTRUCTION DETAILS  
 C8.5



## Area D Keizer

### STORMTECH CHAMBER SPECIFICATIONS

- CHAMBERS SHALL BE STORMTECH MC-4500 OR APPROVED EQUAL.
- CHAMBERS SHALL BE MANUFACTURED FROM VIRGIN, IMPACT MODIFIED POLYPROPYLENE CORPOLYMERS.
- CHAMBER ROWS SHALL PROVIDE CONTINUOUS, UNINTERRUPTED INTERNAL SPACE WITH NO INTERNAL SUPPORT PANELS THAT WOULD IMPIDE FLOW OR LIMIT ACCESS FOR INSPECTION.
- THE STRUCTURAL DESIGN OF THE CHAMBERS, THE STRUCTURAL BACKFILL, AND THE INSTALLATION REQUIREMENTS SHALL ENSURE LONG-DURATION DEAD LOADS AND 2) SHORT-DURATION LIVE LOADS, BASED ON THE AASHTO DESIGN TRUCK WITH CONSIDERATION FOR IMPACT AND MULTIPLE VEHICLE PRESENCE.
- CHAMBERS SHALL MEET THE REQUIREMENTS OF ASTM F2418, "STANDARD SPECIFICATION FOR POLYPROPYLENE (PP) CORRUGATED WALL STORMWATER COLLECTION CHAMBERS".
- CHAMBERS SHALL BE DESIGNED AND ALLOWABLE LOADS DETERMINED IN ACCORDANCE WITH ASTM F2977, "STANDARD PRACTICE FOR STRUCTURAL DESIGN OF THERMOPLASTIC CORRUGATED WALL STORMWATER COLLECTION CHAMBERS".
- ONLY CHAMBERS THAT ARE APPROVED BY THE SITE DESIGN ENGINEER WILL BE ALLOWED. THE CHAMBER MANUFACTURER SHALL SUBMIT THE FOLLOWING UPON REQUEST TO THE SITE DESIGN ENGINEER FOR APPROVAL BEFORE DELIVERING CHAMBERS TO THE PROJECT SITE.
  - A STRUCTURAL EVALUATION SEALED BY A REGISTERED PROFESSIONAL ENGINEER THAT DEMONSTRATES THAT THE SAFETY FACTORS SPECIFIED IN ASTM F2418 AND ASTM F2977 ARE MET FOR LIVE LOADS AND 1) 1/3 FOR LIVE LOADS, THE MINIMUM REQUIRED BY ASTM F2977 AND BY AASHTO FOR THERMOPLASTIC PIPE.
  - A STRUCTURAL EVALUATION SEALED BY A REGISTERED PROFESSIONAL ENGINEER THAT DEMONSTRATES THAT THE LOAD FACTORS SPECIFIED IN THE AASHTO LFD BRIDGE DESIGN SPECIFICATIONS, SECTION 12.12, ARE MET. THE 90 YEAR CREEP MODULUS DATA SPECIFIED IN ASTM F2418 MUST BE USED AS PART OF THE AASHTO STRUCTURAL EVALUATION TO VERIFY LONG TERM PERFORMANCE.
  - STRUCTURAL CROSS SECTION DETAIL ON WHICH THE STRUCTURAL EVALUATION IS BASED.
- CHAMBERS AND END CAPS SHALL BE PRODUCED AT AN ISO 9001 CERTIFIED MANUFACTURING FACILITY.

### IMPORTANT - NOTES FOR THE BIDDING AND INSTALLATION OF MC-4500 CHAMBER SYSTEM

- STORMTECH MC-4500 CHAMBERS SHALL NOT BE INSTALLED UNTIL THE MANUFACTURER'S REPRESENTATIVE HAS COMPLETED A PRE-CONSTRUCTION MEETING WITH THE INSTALLERS. J
  - CHAMBERS ARE NOT TO BE BACKFILLED WITH A DOZER OR EXCAVATOR SITUATED OVER THE CHAMBERS.
  - STORMTECH RECOMMENDS BACKFILL METHODS:
    - STONES/ROCK LOCATED OFF OF THE CHAMBER BED.
    - BACKFILL FROM OUTSIDE THE EXCAVATION USING A LONG BOOM HOE OR EXCAVATOR.
  - THE FOUNDATION STONE SHALL BE LEVELLED AND COMPACTED PRIOR TO PLACING CHAMBERS.
  - JOINTS BETWEEN CHAMBERS SHALL BE PROPERLY SEATED PRIOR TO PLACING STONE.
  - INLET AND OUTLET MANIFOLDS MUST BE INSERTED A MINIMUM OF 12" (300 mm) INTO CHAMBER END CAPS.
  - EMBEDMENT STONE SURROUNDING CHAMBERS MUST BE A CLEAN, CRUSHED, ANGULAR STONE 3/4" (20-50 mm) MEETING THE AASHTO M43 STONE SHALL BE BROUGHT UP EVENLY AROUND CHAMBERS SO AS NOT TO DISTORT THE CHAMBER SHAPE. STONE DEPTHS SHOULD NEVER EXCEED 12" (300 mm). THE STONE SHALL BE PLACED ON ALL TOP, END AND SIDE SURFACES OF THE CHAMBERS TO ANCHOR THE CHAMBERS TO THE FOUNDATION STONE. AASHTO RECOMMENDS THE USE OF "FLEXFORM CATCH IT" INSERTS DURING CONSTRUCTION FOR ALL INLETS TO PROTECT THE SUBSURFACE STORMWATER MANAGEMENT SYSTEM FROM CONSTRUCTION SITE MUDSPIT.
  - NOTES FOR CONSTRUCTION EQUIPMENT
    - STORMTECH MC-4500 CHAMBERS SHALL BE INSTALLED IN ACCORDANCE WITH THE "STORMTECH MC-3500/MC-4500 CONSTRUCTION GUIDE". J
    - THE USE OF EQUIPMENT OVER MC-4500 CHAMBERS IS LIMITED.
      - NO RUBBER TIED LOADER/DUMP TRUCK OR EXCAVATORS ARE ALLOWED UNTIL PROPER FILL DEPTHS ARE REACHED IN ACCORDANCE WITH THE "STORMTECH MC-3500/MC-4500 CONSTRUCTION GUIDE".
      - WEIGHT LIMITS FOR CONSTRUCTION EQUIPMENT CAN BE FOUND IN THE "STORMTECH MC-3500/MC-4500 CONSTRUCTION GUIDE".
    - FILL 30" (900 mm) OF STABILIZED COVER MATERIALS OVER THE CHAMBERS IS REQUIRED FOR DUMP TRUCK TRAVEL OR DUMPING.
- USE OF DOZER TO PUSH EMBEDMENT STONE BETWEEN THE ROWS OF CHAMBERS MAY CAUSE DAMAGE TO CHAMBERS AND IS NOT AN ACCEPTABLE METHOD. ANY CHAMBERS DAMAGED BY USING THE "DUMP AND PUSH" METHOD ARE NOT COVERED UNDER THE STORMTECH STANDARD WARRANTY. J
- CONTACT STORMTECH AT 1-888-892-2864 WITH ANY QUESTIONS ON INSTALLATION REQUIREMENTS OR WEIGHT LIMITS FOR CONSTRUCTION EQUIPMENT.

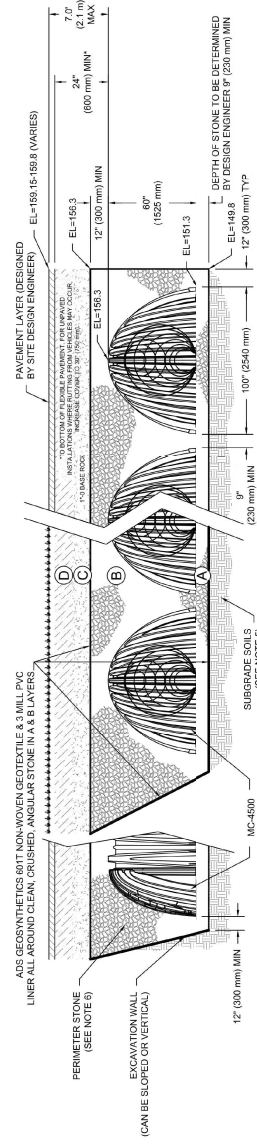
3/20/17 AREA D

### ACCEPTABLE FILL MATERIALS: STORMTECH MC-4500 CHAMBER SYSTEMS

BEGIN COMPACTIONS AFTER 24" (600 mm) OF MATERIAL OVER THE CHAMBERS IS REACHED. COMPACT ADDITIONAL LAYERS IN 12" (300 mm) MAX LIFTS TO A MIN. 95% PROCTOR DENSITY FOR WELL GRADED MATERIAL AND 95% RELATIVE DENSITY FOR PROCESSED AGGREGATE MATERIALS.	CLEAN, CRUSHED, ANGULAR STONE, NOMINAL SIZE DISTRIBUTION BETWEEN 3/4-2 INCH (20-50 mm)	AASHTO M43 <sup>3</sup> 3, 4
INITIAL FILL MATERIAL FOR THE EMBEDMENT STONE (E) SHALL BE PLACED TO ACHIEVE A MIN. 95% PROCTOR DENSITY FOR WELL GRADED MATERIAL AND 95% RELATIVE DENSITY FOR PROCESSED AGGREGATE MATERIALS. GRANULAR FILL FOR ALL SUBGRADE MATERIALS. ~95% PROCTOR DENSITY FOR ALL LAYERS. MOST PAVEMENT SUBGRADE MATERIALS CAN BE USED IN LIEU OF THIS LAYER.	CLEAN, CRUSHED, ANGULAR STONE, NOMINAL SIZE DISTRIBUTION BETWEEN 3/4-2 INCH (20-50 mm)	AASHTO M43 <sup>3</sup> 3, 4 NO COMPACTION REQUIRED.

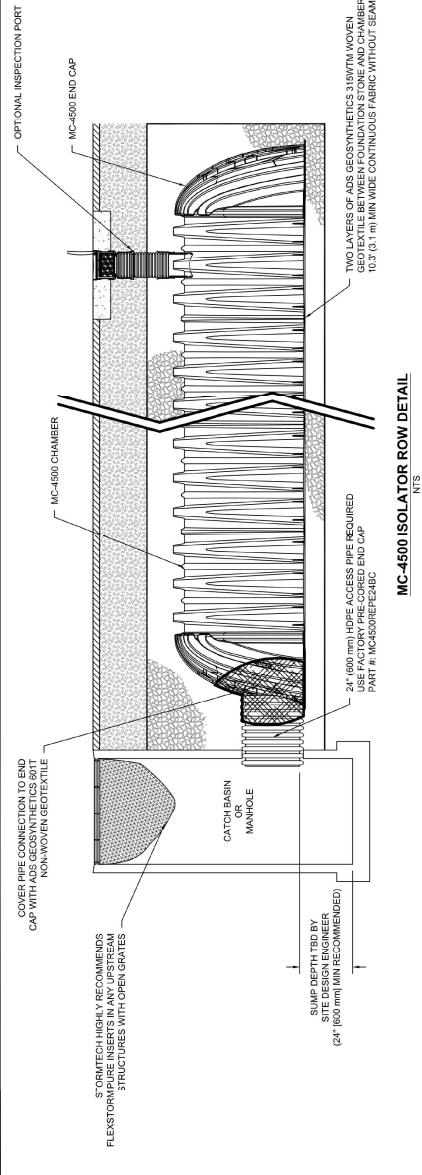
#### PLEASE NOTE:

- THE LISTED AASHTO DESIGNATIONS ARE FOR GRADATIONS ONLY. THE STONE MUST ALSO BE CLEAN, CRUSHED, ANGULAR, FOR EXAMPLE, A SPECIFICATION FOR #4 STONE WOULD STATE "CLEAN, CRUSHED, ANGULAR STONE #4".
- STORMTECH COMPACTON REQUIREMENTS ARE MET FOR A LOCATION MATERIALS WHEN PLACED AND COMPACTED IN 6" (250 mm) MAX LIFTS USING TWO FULL COVERAGES WITH A VIBRATORY COMPACTOR.
- WHERE INFILTRATION SURFACES MAY BE COMPROMISED BY COMPACTION OR STANDARD DESIGN LOAD CONDITIONS, A FLAT SURFACE MAY BE ACHIEVED BY RAKING OR DRAGGING WITHOUT COMPACTION EQUIPMENT. FOR SPECIAL LOAD DESIGN, CONTACT STORMTECH FOR COMPACTON RECOMMENDATIONS.



#### NOTES:

- MC-4500 CHAMBERS SHALL CONFORM TO THE REQUIREMENTS OF ASTM F2418 "STANDARD SPECIFICATION FOR POLYPROPYLENE (PP) CORRUGATED WALL STORMWATER COLLECTION CHAMBERS". J
- MC-4500 CHAMBERS SHALL BE DESIGNED IN ACCORDANCE WITH ASTM F2977, "STANDARD PRACTICE FOR STRUCTURAL DESIGN OF THERMOPLASTIC CORRUGATED WALL STORMWATER COLLECTION CHAMBERS". J
- ACCEPTABLE FILL MATERIALS TABLE ABOVE PROVIDES MATERIAL LOCATION, DESCRIPTIONS, GRADATIONS, AND COMPACTON REQUIREMENTS FOR FOUNDATION, EMBEDMENT, AND FILL MATERIALS. J
- THE SITE DESIGN ENGINEER IS RESPONSIBLE FOR ASSESSING THE BEARING RESISTANCE (ALLOWABLE BEARING CAPACITY) OF THE SUBGRADE SOILS AND THE DEPTH OF FOUNDATION STONE WITH CONSIDERATION FOR THE RANGE OF EXPECTED SOIL MOISTURE CONDITIONS. J
- PERIMETER STONE MUST BE EMBEDDED THROUGH WALL TO THE EXCAVATION WALL FOR BOTH VERTICAL AND SLOPED EXCAVATION WALLS.
- ONCE LAYER 'C' IS PLACED, ANY SUBGRADE CAN BE PLACED IN LAYER 'D' UP TO THE FINISHED GRADE. MOST PAVEMENT SUBGRADE SOILS CAN BE USED TO REPLACE THE MATERIAL REQUIREMENTS OF LAYER 'C'.
- ON 'B' AT THE SITE DESIGN ENGINEER'S DISCRETION.



#### INSPECTION & MAINTENANCE

- STEP 1) INSPECT FOR THE FOLLOWING:
- REMOVE/OPEN ID ON NYLON PLAST HINGED DRAIN BODY W/ SOLID HINGED PART 27/24/009A
  - USING A FLASHLIGHT AND STADIUM ROD, MEASURE DEPTH OF SEDIMENT AND RECORD ON MAINTENANCE LOG
  - LOWER A CAMERA INTO ISOLATOR ROW FOR VISUAL INSPECTION OF SEDIMENT LEVELS (OPTIONAL)
- ALL ISOLATOR ROWS AT, OR ABOVE, 3" (76 mm) PROCEED TO STEP 2. IF NOT, PROCEED TO STEP 3.
- REMOVE COVER FROM STRUCTURE AT UPSTREAM END OF ISOLATOR ROW. INSURE ALL ISOLATOR OR SPACE ENTRY IS USED TO AVOID A CONFINED SPACE ENTRY. FOLLOW OSHA REGULATIONS FOR CONFINED SPACE ENTRY IF ENTERING MANHOLE.
  - IF SEDIMENT IS AT, OR ABOVE, 3" (76 mm) PROCEED TO STEP 2. IF NOT, PROCEED TO STEP 3.
- STEP 2) CLEAN OUT ISOLATOR ROW USING THE JETVAC PROCESS.
- APPLY MULTIPLE PASSES OF JETVAC UNTIL BACKLUSH WATER IS CLEAN.
  - VACUUM STRUCTURE SUMP AS REQUIRED.
- STEP 3) REPLACE ALL COVERS, GRATES, FILTERS, AND LIDDS. RECORD OBSERVATIONS AND ACTIONS.
- STEP 4) INSPECT AND CLEAN BASINS AND MANHOLES UPSTREAM OF THE STORMTECH SYSTEM.

#### NOTES

- INSPECT EVERY 6 MONTHS DURING THE FIRST YEARS OF OPERATION. ADJUST THE INSPECTION INTERVAL BASED ON PREVIOUS OBSERVATIONS OF SEDIMENT ACCUMULATION AND HIGH WATER ELEVATIONS. J
- CONDUCT JETVAC AND VACUUMING ANNUALLY OR WHEN INSPECTION SHOWS THAT MAINTENANCE IS NECESSARY.

WESTTECH ENGINEERING, INC.  
CONSULTING ENGINEERS AND PLANNERS

3841 Edinboro Industrial Dr., S.E., Suite 100, Salem, OR 97302  
Phone: (503) 562-2474 Fax: (503) 565-3988  
E-mail: westtech@westtech-eng.com

WE # : 31.50.0000.0

## AS-BUILT

COMPILED FROM:

FINAL PROOF SURVEY \_\_\_\_\_  
DATA FURNISHED BY CONTRACTOR  X   
FIELD INSPECTION RECORDS \_\_\_\_\_

WESTTECH ENGINEERING, INC.

DATE: 10.1.20 BY: AK

C8.6

CONSTRUCTION  
DETAILS

**BSAA**  
ARCHITECTURE & PLANNING

BENNER  
STANGE  
ASSOCIATES  
ARCHITECTS, INC.

THE WATERMAN BUILDING  
80 SE MADISON STREET  
SUITE 430  
PORTLAND, OR 97214  
503-670-0234  
FAX 503-670-0234  
bsa@bsaarch.com

PROJECT NO. \_\_\_\_\_

DRAWN BY :  
AK

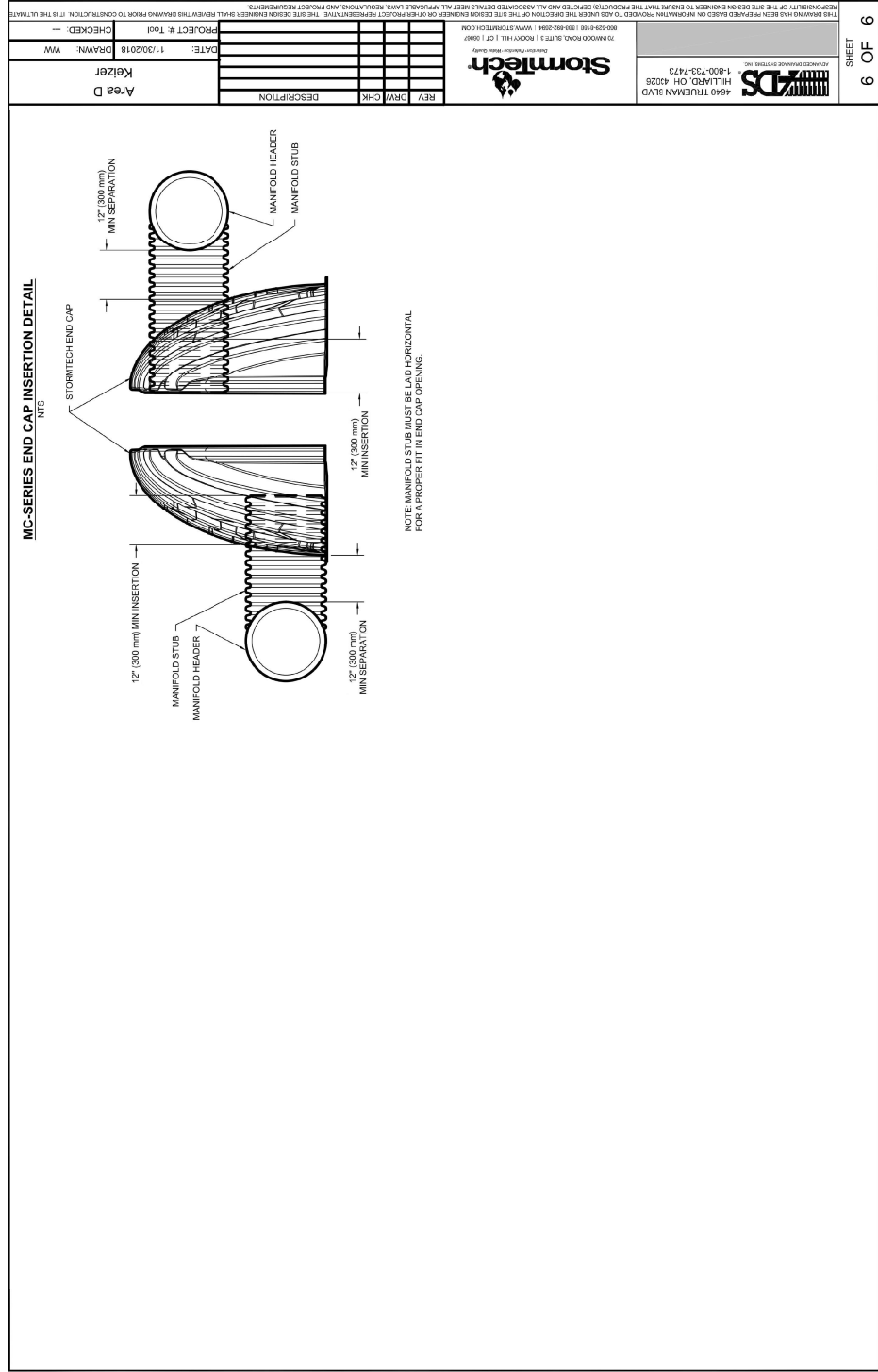
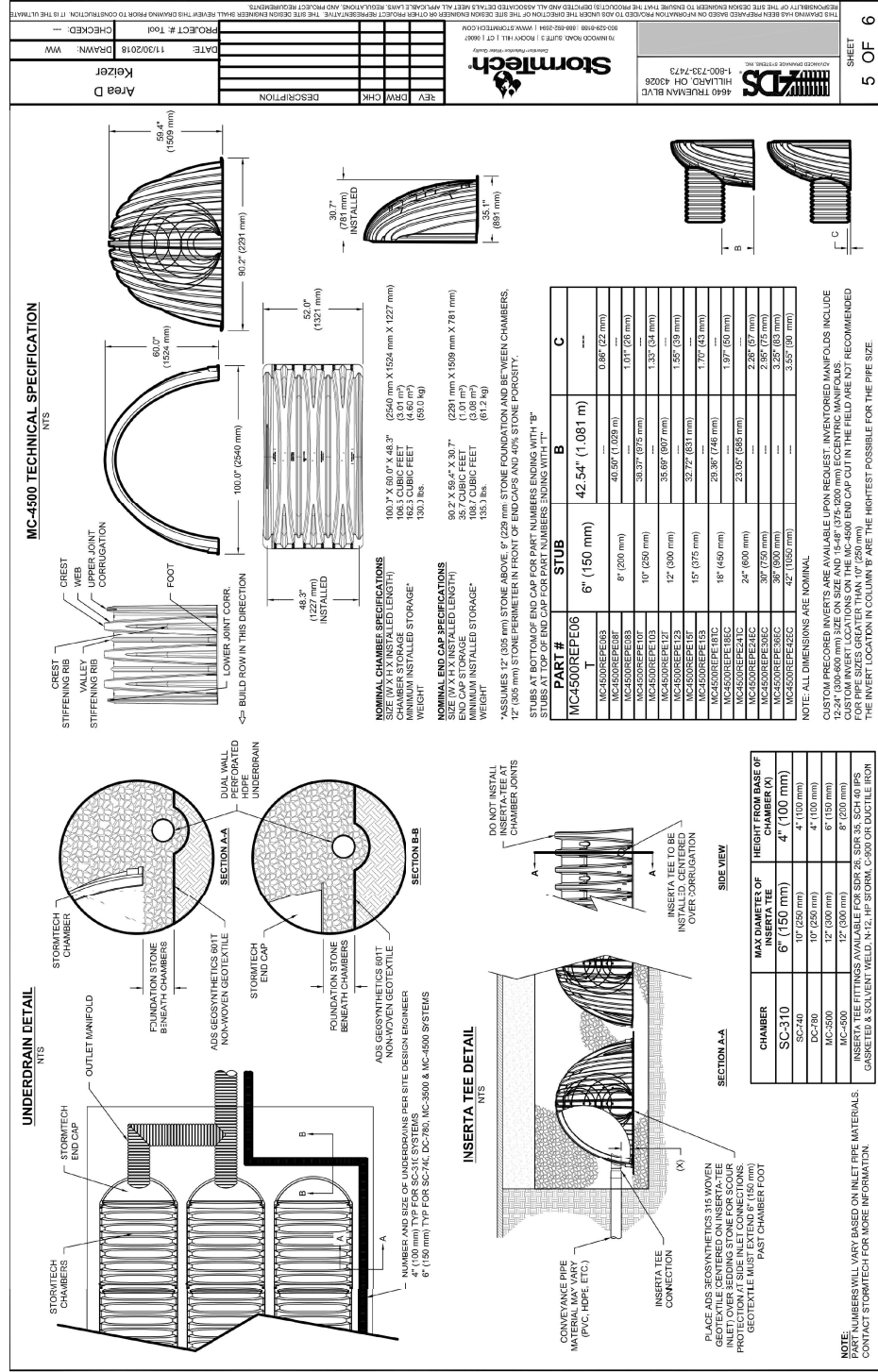
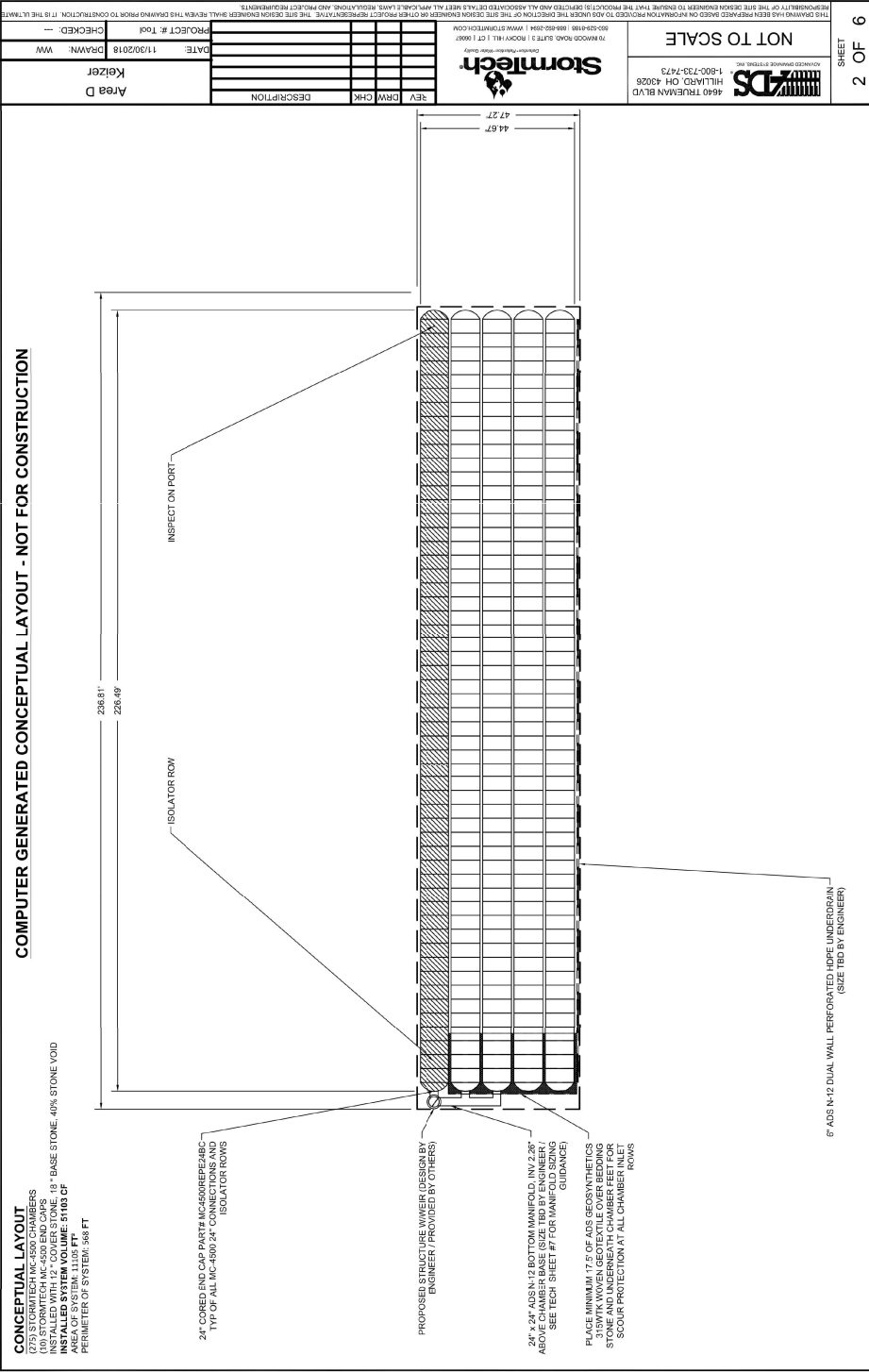
CHECKED BY :  
JW

DATE :  
6-12-19

REVISION :  
1. 7.22.19 AK  
PER CITY COMMENTS  
2.  
3.  
4.  
5.  
6.

Chemawa Station - Area 'D'  
Ulali Drive | Keizer, Oregon  
Chemawa Station LLC





THIS DRAWING HAS BEEN PREPARED BASED ON INFORMATION PROVIDED TO THE DESIGN ENGINEER BY THE CLIENT. THE DESIGN ENGINEER SHALL VERIFY THE DESIGN ENGINEER'S RESPONSIBILITY OF THE DESIGN ENGINEER TO ENSURE THAT THE PROJECT'S DESIGN MEETS ALL APPLICABLE LOCAL, STATE, AND FEDERAL REGULATIONS AND PROJECT REQUIREMENTS. IT IS THE USER'S RESPONSIBILITY TO OBTAIN ALL NECESSARY PERMITS AND APPROVALS FROM THE APPROPRIATE AGENCIES.

PROJECT # 1001  
 DATE: 11/30/2018  
 DRAWN: WW  
 CHECKED: --

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PROJECT # 1001  
 DATE: 11/30/2018  
 DRAWN: WW  
 CHECKED: --

**AS-BUILT**  
 COMPILED FROM:  
 FINAL PROOF SURVEY \_\_\_\_\_  
 DATA FURNISHED BY CONTRACTOR  X   
 FIELD INSPECTION RECORDS \_\_\_\_\_  
**WESTTECH ENGINEERING, INC.**  
 DATE:  10.1.20  BY:  AK

**WESTTECH ENGINEERING, INC.**  
 CONSULTING ENGINEERS AND PLANNERS  
 3841 Edinboro Industrial Dr., S.E., Suite 100, Salem, OR 97302  
 Phone: (503) 586-2474 Fax: (503) 586-3988  
 E-mail: westtech@westtech-eng.com  
 WE #: 31.500.000.0



**Chemawa Station - Area 'D'**  
 Ulali Drive | Keizer, Oregon  
 Chemawa Station LLC

PROJECT NO. \_\_\_\_\_  
 DRAWN BY: AK  
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 1. 7.22.19 AK  
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CONSTRUCTION  
 DETAILS  
**C8.7**



## **Westtech Engineering, Inc.**

3841 Fairview Industrial Drive SE, Suite 100

Salem, OR 97302

(503) 585-2474 FAX: (503) 585-3986

August 6, 2019

City of Keizer  
930 Chemawa Rd NE  
Keizer, OR 97303

RE: Stormwater Calculations – Chemawa Station, LLC  
JO# 3150.0000.0

To Whom It May Concern:

Westtech Engineering submits this Stormwater Calculations Summary for the Chemawa Station development project in Keizer, Oregon.

The remainder of this letter is divided into the following sections:

- Project Overview
- Summary of Methods
- Analysis Results
- Stormwater Basin Map & Water Quality Basin Map
- Keizer Station Basin & Flow Map
- Appendix A – HydroCAD Model Analysis
- Appendix B – Geotechnical Report

Short discussions on these items follow.

### **Project Overview and Existing Conditions**

The proposed project is located just south of Keizer Station, bounded between Ulali Dr to the south and Chemawa Road to the north (Refer to Stormwater Basin Map, Basin A). The project scope is to develop a commercial retail shopping center with associated utilities, parking and provide detention for the entire Chemawa Station property (drainage area is approximately 21.69 acres, Basin A, B & C). Refer to the Civil Drawings for a site map of the project area.

The existing site is located within the Keizer Station management area as shown on the attached Keizer Station Stormwater Basin Map and Allowed Developed Flows. The allowed release rates to the Keizer Station stormwater system are as follows in Table 1.



**Table 1 – Keizer Station Post Developed Allowable Detention Release Rates**

Flow Return Storm Event	Allowable Detention Release Rates (CFS)
5 yr	4.48
10 yr	6.05
25 yr	8.00
50 yr	9.82
100 yr	10.22

This project will fill the existing detention pond located in Basin A and replace it with an underground detention system that will provide detention for both Basin A, Basin B, and the undetained Basin C at full buildout.

## Summary of Methods

### Software Used

HydroCAD modeling software was used to size the stormwater detention facility. The Santa Barbara Unit Hydrograph Type 1A storm was used to model the design storm, the same as the Keizer Station Stormwater Master Plan. HydroCAD modeling analysis is included in Appendix A.

### 24 hour Rainfall Depths

The 24-hour storm depths used for the model were the same depths used in the Keizer Station Master Plan. These are shown as follows in Table 2.

**Table 2 – Keizer Station Master Plan 24hr Storm Depths**

Flow Return Storm Event	Storm Depth (inches)
Water Quality	1.05
5 yr	3.0
10 yr	3.45
25 yr	3.98
50 yr	4.45
100 yr	4.55

### Curve Numbers and Time of Concentration

Developed curve numbers of 98 and 74 were used for the buildings/impervious surface and landscaping, respectively. These CN's are consistent with the Keizer Station Master Plan

A minimum time of concentration of 10 minutes is applied to the developed basins in order to be consistent with the Keizer Station Stormwater Master Plan.

### Drainage Basin

Refer to the Stormwater Basin Map. There are three developed (at buildout) drainage basins as shown in Table 3.

**Table 3 - Drainage Basin Areas**

Drainage Basin	Impervious Area (AC)	Pervious Area (AC)
Basin A	5.87	1.03
Basin West*	1.76	0
Basin East*	0.92	0
Basin B	12.07	2.13
Basin C	0.50	0.09

\*Assumes the entire basin is impervious to be conservative.

## Analysis Results

### Detention

The new underground detention facility is designed to capture the post developed flows from Basin A and Basin B and release the post developed flows at the release rates allowed in the Keizer Station Master Plan as shown in Table 1. A summary of allowed and proposed release rates, detention volumes and elevations are shown below in Table 4.

**Table 4 – Proposed Underground Detention Facility Summary**

Storm Event	Allowable Detention Release Rates (CFS)	Provided Detention Release Rates (CFS)	Water Surface Elevation/Volume (Ft/CF)*	Orifice Elevation/Size (Ft/Inches)
5 yr	4.48	4.46	153.92 / 29,682	149.11/8.7”
10 yr	6.05	6.03	154.64 / 35,379	153.84/7.5”
25 yr	8.00	7.98	155.58 / 41,893	154.54/6.3”
50 yr	9.82	9.82	156.85 / 47,859	155.41 /3”
100 yr	10.22	10.22	157.15 / 49,132	none

\*The bottom of the underground detention is at elevation 149.8.

As shown in the above table and the attached model results in Appendix A the new underground detention facility has sufficient capacity to detain the build out flows of Basin A and B and meet the Keizer Station Master Plan release rates.

### Water Quality

In accordance with the City’s MSR Permit, we are required to treat the runoff from the 1.05-inch storm event. Due to the site compaction that was completed to the site prior, infiltration facilities

are not feasible. However, we are proposing to use Oldcastle Perfilters (Approved by Washington DOE) placed in two manholes to provide stormwater quality treatment for the proposed development.

**Table 5 – Proposed Water Quality Treatment Facility Summary**

<b>Drainage Basin</b>	<b>Water Quality Flows (cfs)</b>	<b>Provided Water Quality Flows (cfs)</b>
Basin West	0.38	0.42
Basin East	0.20	0.20

As shown in the Table 5, we are proposing to meet or exceed the required treatment flow rates.

If you have any questions or need additional information regarding our Stormwater Calculations, please contact us at (503) 585-2474.

Sincerely,

**WESTECH ENGINEERING, INC.**

  
\_\_\_\_\_  
W. Josh Wells, P.E.



RENEWS: 6/30/2020

# Keizer Station Area D Basin Map

Write a description for your map.

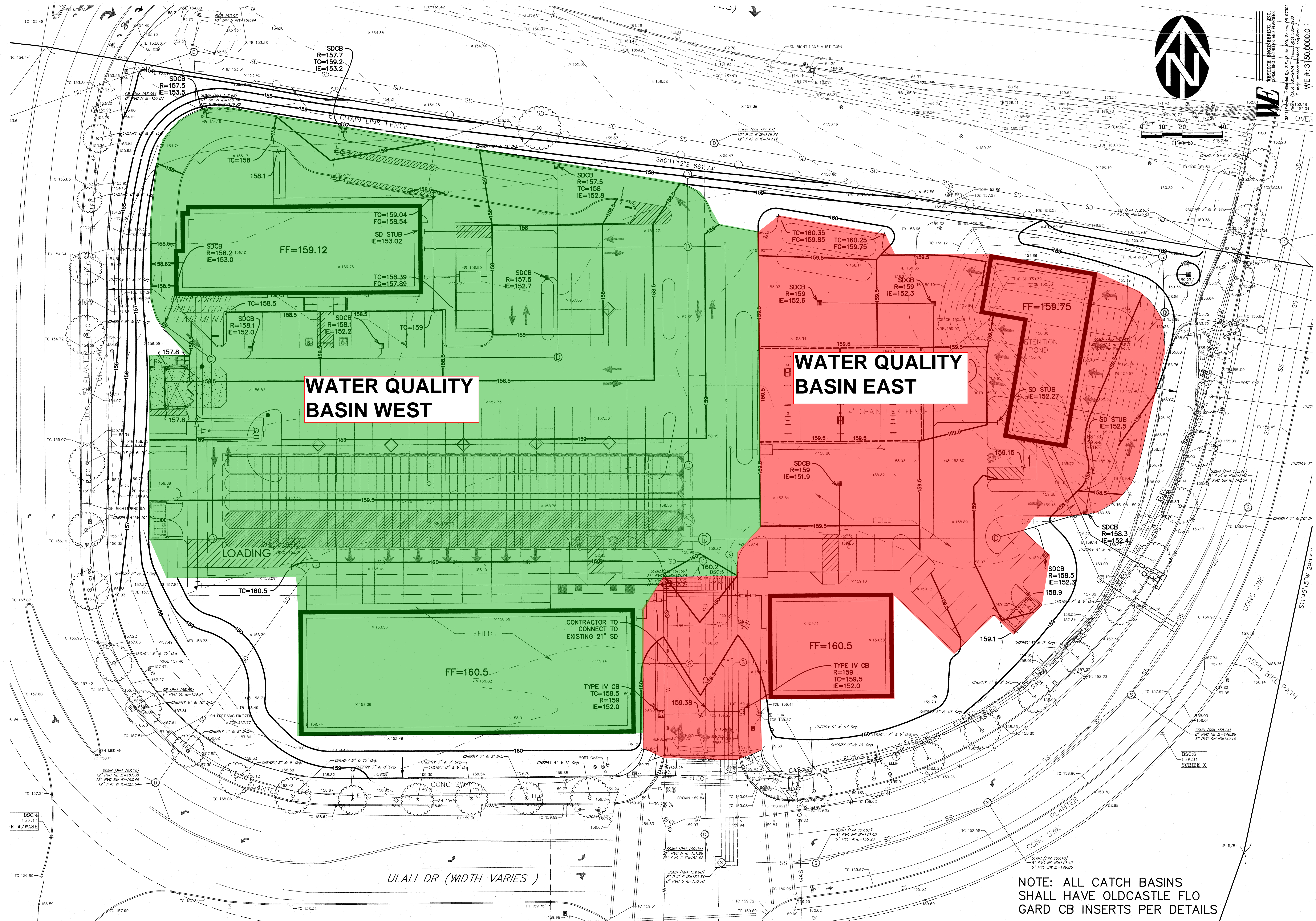
Legend



**BASIN B**  
Area = 14.2 AC

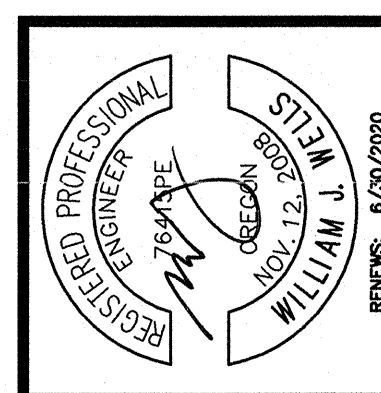
**BASIN A**  
Area = 6.90 AC

**BASIN C**  
Area = 0.59 AC



**WATER QUALITY  
BASIN WEST**

**WATER QUALITY  
BASIN EAST**



**Chemawa Station - Area 'D'**  
Ulali Drive | Keizer, Oregon  
Chemawa Station LLC

**PROJECT NO.**  
**DRAWN BY :**  
AK  
**CHECKED BY :**  
JW  
**DATE :**  
6-12-19  
**REVISION :**

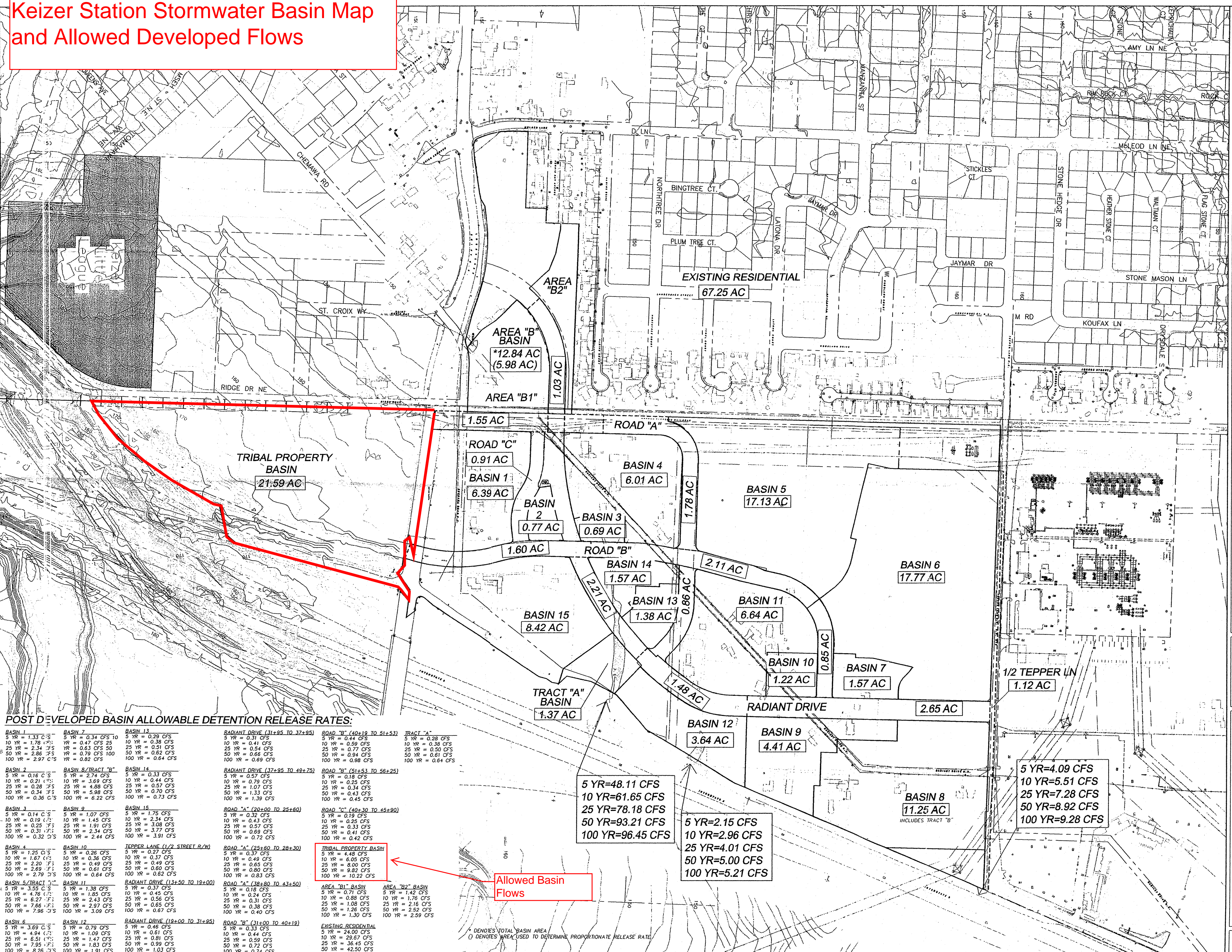
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**OVERALL GRADING PLAN**  
**C4.0**

**NOTE: ALL CATCH BASINS SHALL HAVE OLDCASTLE FLO GARD CB INSERTS PER DETAILS**

# Keizer Station Stormwater Basin Map and Allowed Developed Flows

DWG. NO.: 30772-001-000  
 DATE: 11/15/04  
 DRAWN BY: J. GREGORY  
 CHECKED BY: J. GREGORY  
 PLOT DATE: 11/15/04



### POST DEVELOPED BASIN ALLOWABLE DETENTION RELEASE RATES:

<b>BASIN 1</b> 5 YR = 1.33 CFS 10 YR = 1.78 CFS 25 YR = 2.34 CFS 50 YR = 2.86 CFS 100 YR = 2.97 CFS	<b>BASIN 7</b> 5 YR = 0.34 CFS 10 YR = 0.47 CFS 25 YR = 0.63 CFS 50 YR = 0.79 CFS 100 YR = 0.82 CFS	<b>BASIN 13</b> 5 YR = 0.29 CFS 10 YR = 0.38 CFS 25 YR = 0.51 CFS 50 YR = 0.62 CFS 100 YR = 0.64 CFS	<b>RADIANT DRIVE (31+95 TO 37+95)</b> 5 YR = 0.57 CFS 10 YR = 0.79 CFS 25 YR = 1.07 CFS 50 YR = 1.33 CFS 100 YR = 1.39 CFS	<b>ROAD "B" (40+19 TO 51+53)</b> 5 YR = 0.44 CFS 10 YR = 0.59 CFS 25 YR = 0.77 CFS 50 YR = 0.94 CFS 100 YR = 0.98 CFS	<b>TRACT "A"</b> 5 YR = 0.28 CFS 10 YR = 0.38 CFS 25 YR = 0.50 CFS 50 YR = 0.61 CFS 100 YR = 0.64 CFS
<b>BASIN 2</b> 5 YR = 0.16 CFS 10 YR = 0.21 CFS 25 YR = 0.28 CFS 50 YR = 0.34 CFS 100 YR = 0.36 CFS	<b>BASIN 8/TRACT "B"</b> 5 YR = 2.74 CFS 10 YR = 3.69 CFS 25 YR = 4.88 CFS 50 YR = 5.98 CFS 100 YR = 6.22 CFS	<b>BASIN 14</b> 5 YR = 0.33 CFS 10 YR = 0.44 CFS 25 YR = 0.57 CFS 50 YR = 0.70 CFS 100 YR = 0.73 CFS	<b>RADIANT DRIVE (37+95 TO 49+75)</b> 5 YR = 0.57 CFS 10 YR = 0.79 CFS 25 YR = 1.07 CFS 50 YR = 1.33 CFS 100 YR = 1.39 CFS	<b>ROAD "B" (51+53 TO 56+25)</b> 5 YR = 0.18 CFS 10 YR = 0.25 CFS 25 YR = 0.34 CFS 50 YR = 0.43 CFS 100 YR = 0.45 CFS	
<b>BASIN 3</b> 5 YR = 0.14 CFS 10 YR = 0.19 CFS 25 YR = 0.25 CFS 50 YR = 0.31 CFS 100 YR = 0.32 CFS	<b>BASIN 9</b> 5 YR = 1.07 CFS 10 YR = 1.45 CFS 25 YR = 1.91 CFS 50 YR = 2.34 CFS 100 YR = 2.44 CFS	<b>BASIN 15</b> 5 YR = 1.75 CFS 10 YR = 2.34 CFS 25 YR = 3.17 CFS 50 YR = 3.77 CFS 100 YR = 3.91 CFS	<b>ROAD "A" (20+00 TO 25+60)</b> 5 YR = 0.32 CFS 10 YR = 0.43 CFS 25 YR = 0.57 CFS 50 YR = 0.69 CFS 100 YR = 0.72 CFS	<b>ROAD "C" (40+30 TO 45+90)</b> 5 YR = 0.32 CFS 10 YR = 0.43 CFS 25 YR = 0.57 CFS 50 YR = 0.69 CFS 100 YR = 0.72 CFS	
<b>BASIN 4</b> 5 YR = 1.25 CFS 10 YR = 1.67 CFS 25 YR = 2.20 CFS 50 YR = 2.69 CFS 100 YR = 2.79 CFS	<b>BASIN 10</b> 5 YR = 0.26 CFS 10 YR = 0.36 CFS 25 YR = 0.49 CFS 50 YR = 0.61 CFS 100 YR = 0.64 CFS	<b>TEPPER LANE (1/2 STREET R/W)</b> 5 YR = 0.27 CFS 10 YR = 0.37 CFS 25 YR = 0.49 CFS 50 YR = 0.60 CFS 100 YR = 0.62 CFS	<b>ROAD "A" (25+60 TO 28+30)</b> 5 YR = 0.49 CFS 10 YR = 0.65 CFS 25 YR = 0.80 CFS 50 YR = 0.93 CFS 100 YR = 0.93 CFS	<b>TRIBUTARY BASIN</b> 5 YR = 0.49 CFS 10 YR = 0.65 CFS 25 YR = 0.80 CFS 50 YR = 0.93 CFS 100 YR = 0.93 CFS	
<b>BASIN 5/TRACT "A"</b> 5 YR = 3.55 CFS 10 YR = 4.76 CFS 25 YR = 6.27 CFS 50 YR = 7.66 CFS 100 YR = 7.96 CFS	<b>BASIN 11</b> 5 YR = 1.38 CFS 10 YR = 1.85 CFS 25 YR = 2.43 CFS 50 YR = 2.97 CFS 100 YR = 3.09 CFS	<b>RADIANT DRIVE (13+50 TO 19+00)</b> 5 YR = 0.45 CFS 10 YR = 0.61 CFS 25 YR = 0.80 CFS 50 YR = 0.97 CFS 100 YR = 1.03 CFS	<b>ROAD "A" (38+80 TO 43+50)</b> 5 YR = 0.37 CFS 10 YR = 0.49 CFS 25 YR = 0.63 CFS 50 YR = 0.76 CFS 100 YR = 0.77 CFS	<b>AREA "B1" BASIN</b> 5 YR = 0.71 CFS 10 YR = 0.95 CFS 25 YR = 1.26 CFS 50 YR = 1.58 CFS 100 YR = 1.63 CFS	
<b>BASIN 6</b> 5 YR = 3.69 CFS 10 YR = 4.94 CFS 25 YR = 6.51 CFS 50 YR = 7.95 CFS 100 YR = 8.26 CFS	<b>BASIN 12</b> 5 YR = 0.79 CFS 10 YR = 1.09 CFS 25 YR = 1.47 CFS 50 YR = 1.83 CFS 100 YR = 1.91 CFS	<b>RADIANT DRIVE (19+00 TO 31+95)</b> 5 YR = 0.46 CFS 10 YR = 0.62 CFS 25 YR = 0.81 CFS 50 YR = 0.99 CFS 100 YR = 1.03 CFS	<b>ROAD "B" (31+00 TO 40+19)</b> 5 YR = 0.33 CFS 10 YR = 0.44 CFS 25 YR = 0.59 CFS 50 YR = 0.72 CFS 100 YR = 0.74 CFS	<b>EXISTING RESIDENTIAL</b> 5 YR = 24.00 CFS 10 YR = 29.67 CFS 25 YR = 36.45 CFS 50 YR = 42.50 CFS 100 YR = 43.80 CFS	

5 YR=48.11 CFS  
 10 YR=61.65 CFS  
 25 YR=78.18 CFS  
 50 YR=93.21 CFS  
 100 YR=96.45 CFS

5 YR=2.15 CFS  
 10 YR=2.96 CFS  
 25 YR=4.01 CFS  
 50 YR=5.00 CFS  
 100 YR=5.21 CFS

5 YR=4.09 CFS  
 10 YR=5.51 CFS  
 25 YR=7.28 CFS  
 50 YR=8.92 CFS  
 100 YR=9.28 CFS

Allowed Basin Flows

\* DENOTES TOTAL BASIN AREA  
 (1) DENOTES AREA USED TO DETERMINE PROPORTIONATE RELEASE RATE

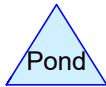
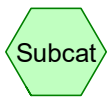
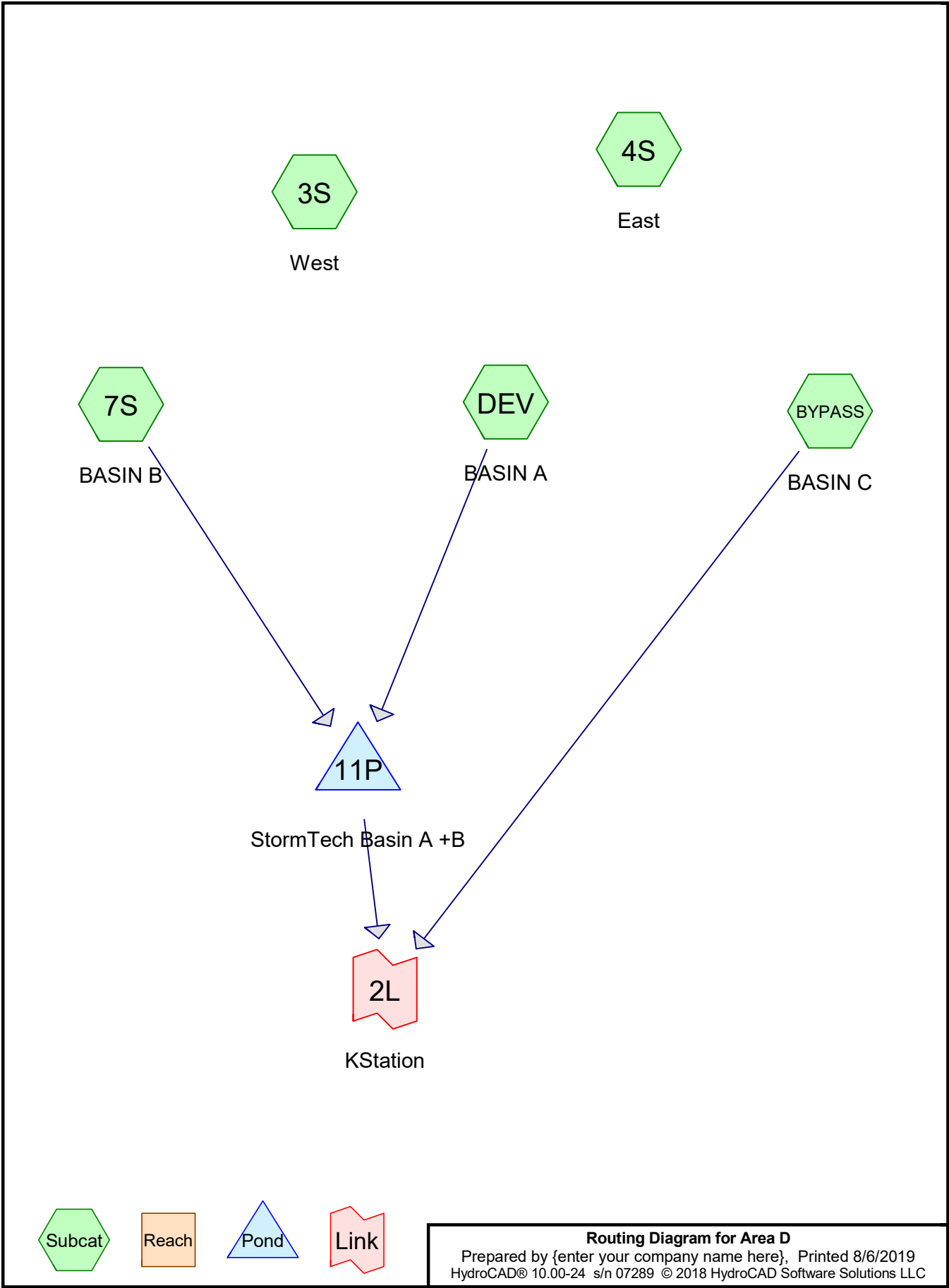
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NORTHWEST NATIONAL LLC  
 KEIZER STATION VILLAGE CENTER  
 DEVELOPED CONDITIONS  
 DRAINAGE BASIN MAP/DETENTION RELEASE RATES

PROJECT NO. 30772  
 DRAWING FILE NAME: 30772-land-ex01  
 SCALE: 1" = 160'

CHECKED BY: JRM  
 APPROVED BY: JRM  
 DATE: 11/15/04  
 PLOT DATE: 11/15/04  
 REVISION: 01

SHEET  
**EX03**



**Routing Diagram for Area D**  
 Prepared by {enter your company name here}, Printed 8/6/2019  
 HydroCAD® 10.00-24 s/n 07289 © 2018 HydroCAD Software Solutions LLC